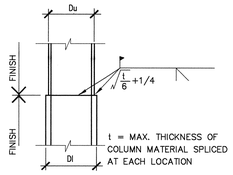
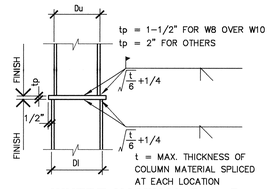


1 WHERE NOMINAL Du = DI TYPICAL COLUMN SPLICE

N.T.S.



2 WHERE NOMINAL Du < DI TYPICAL COLUMN SPLICE

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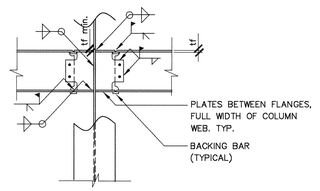
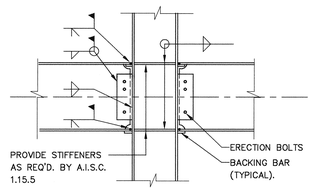


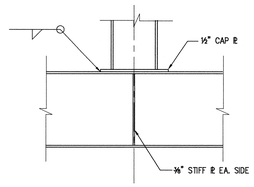
3 TYPICAL BEAM TO COLUMN MOMENT CONNECTION AT WEB (WELDED)

N.T.S.



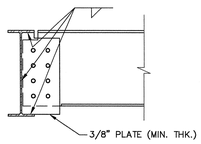
4 TYPICAL BEAM TO COLUMN MOMENT CONNECTION AT FLANGE (WELDED)

N.T.S.



5 TYPICAL DETAIL - COLUMN SUPPORTED ON BEAM

N.T.S.



6 SKEWED BEAM TO BEAM CONNECTION

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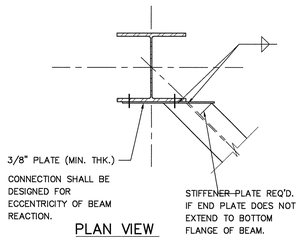
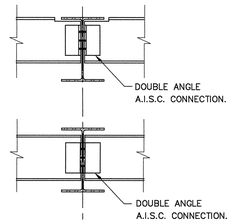


7 SKEWED BEAM TO COLUMN CONNECTION

N.T.S.



8 TYPICAL BEAM TO BEAM SHEAR CONNECTION

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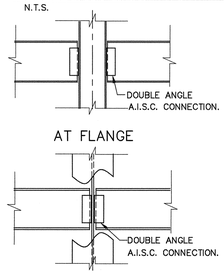

9 TYPICAL BEAM TO COLUMN SHEAR CONNECTION

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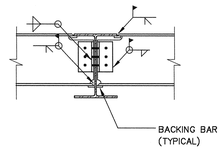
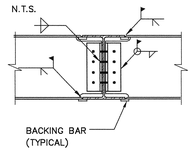


10 UNEQUAL DEPTH BEAMS MOMENT CONNECTION



11 EQUAL DEPTH BEAMS MOMENT CONNECTION

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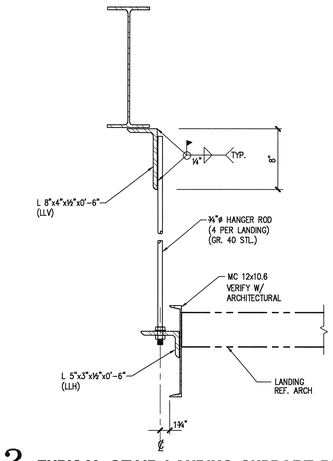
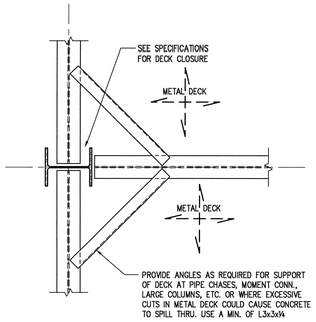
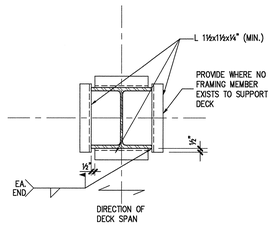


12 TYPICAL STAIR LANDING SUPPORT DETAIL



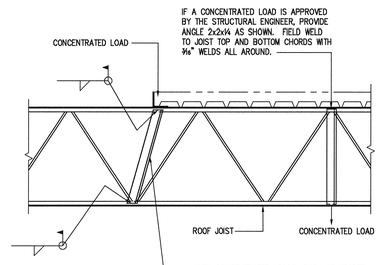
13 TYPICAL DETAIL FOR SUPPORT OF METAL DECK AT COLUMNS

N.T.S.



14 DECK SUPPORT ANGLE @ STL COL.

N.T.S.



15

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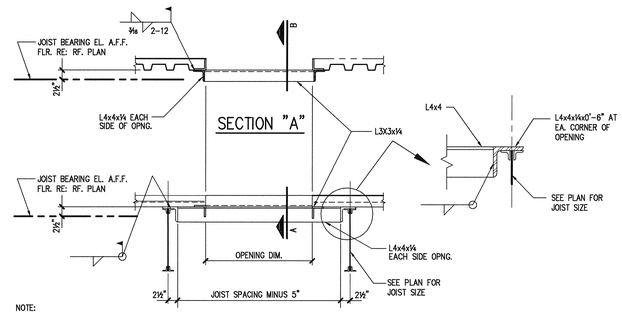
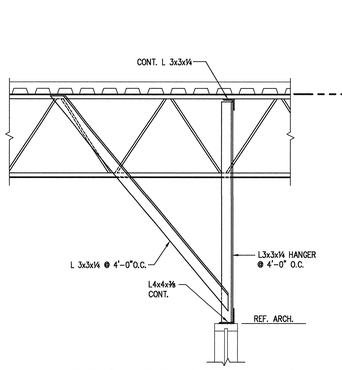
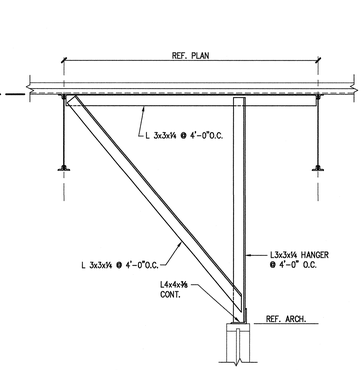


16 TYPICAL OPENING FRAMING DETAIL

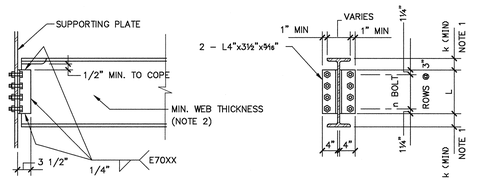
N.T.S.



17 TYPICAL DETAIL AT HEAD SUPPORT FOR WINDOW WALL PERPENDICULAR TO JOIST



18 TYPICAL DETAIL AT HEAD SUPPORT FOR WINDOW WALL PARALLEL TO JOIST



"n" NUMBER OF ROWS	L (INCHES)	ALLOWABLE BOLT CAPACITY (KIPS)	ALLOWABLE WELD CAPACITY (KIPS)
2	5 1/2	37	48
3	8 1/2	56	77
4	11 1/2	74	100
5	14 1/2	93	122
6	17 1/2	111	143
7	20 1/2	130	160
8	23 1/2	148	180
9	26 1/2	167	198
10	29 1/2	186	215

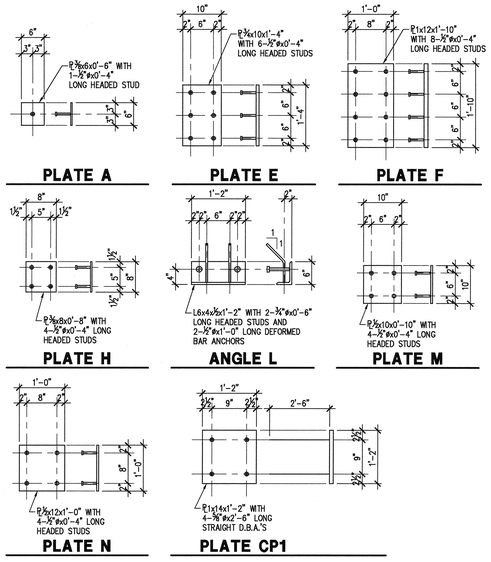
NOTES:

1. REFER TO AISC – MANUAL OF STEEL CONSTRUCTION (SECTION 1).
2. ALLOWABLE WELD CAPACITY IS BASED UPON A BEAM WEB THICKNESS OF 1/2" FOR A36 MATERIAL AND 3/8" FOR ASTM A572, GRADE 50 MATERIAL. REDUCE THIS CAPACITY PROPORTIONALLY FOR A LESSER WEB THICKNESS.
3. THE SUPPORTING PLATE CAPACITY SHALL BE BASED UPON AN ALLOWABLE LOAD PER BOLT OF 65 KIPS PER INCH OF PLATE THICKNESS FOR A36 MATERIAL OR 73 KIPS PER INCH OF PLATE THICKNESS FOR ASTM A572, GRADE 50 MATERIAL. FOR BEAM CONNECTIONS ON TWO SIDES, THE SUM OF THE LOADS PER BOLT SHALL BE CONSIDERED.
4. FOR COPED BEAM CONNECTIONS, THE CAPACITY OF THE NET SHEAR AREA OF THE WEB SHALL BE VERIFIED.
5. THE CAPACITY OF THE CONNECTION SHALL BE THE LESSER VALUE OF THE ALLOWABLE BOLT CAPACITY, ALLOWABLE WELD CAPACITY, SUPPORTING PLATE CAPACITY OR THE WEB NET SHEAR AREA CAPACITY.
6. THE MINIMUM NUMBER OF ROWS OF BOLTS SHALL BE AS FOLLOWS:

W10 & W12	2 ROWS
W14 & W16	3 ROWS
W18, W21, & W24	4 ROWS
W27 & W30	5 ROWS
W33 & W36	6 ROWS

19 STANDARD BEAM CONNECTION DETAIL

3/4" ø A325 BOLTS – BEARING CONN. – STANDARD HOLES



20 EMBED PLATES

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LADERA BEND

PACKAGE B
RETAIL BUILDING

Project	16-5236-00
Drawn	CJR
Quality Control	
Project Director	

Issues
1 CONSTRUCTION 02-07-07

STANDARD STEEL DETAILS

S6.0